

Mat-2.177 Seminar on case studies in
operations research

Decision-Making support system for flood control

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7.6.2004

Contents

- 1 Introduction 4**
 - 1.1 Background 4
 - 1.1.1 Client - EIA 4
 - 1.1.2 Case - Yangtze River flood modeling 4
 - 1.1.3 Assignment and objectives 4
 - 1.1.4 Structure of the project 5
 - 1.1.5 Project execution 6
 - 1.2 Literature review 7
 - 1.3 Structure of the report 8

- 2 Decision situation 9**

- 3 Our approach 10**
 - 3.1 Problem formulation 11
 - 3.2 Restrictions 12

- 4 Discussion 13**
 - 4.1 The dynamics of the model 13
 - 4.2 Estimating the flow distributions 14
 - 4.3 Calculating the risk levels 14

- 5 Case Yangtze 14**
 - 5.1 Data analysis 14
 - 5.2 Method demonstration 16

1 Introduction

1.1 Background

1.1.1 Client - EIA

Environmental Impact Assessment Centre of Finland Ltd. (EIA) is an independent research company. The main aim of the company is to apply state of the art mathematical models for practical demands, and to maintain the validity of models with their continuous development and critical evaluation against field observations. EIA models have been developed for watersheds, rivers, lakes, coastal and sea areas and atmosphere.

This project concerned watershed management and river flood risk analysis. Risk analyses of floods and decision-making support systems for watershed management have become even more important as hydroelectric power plants have become more common.

1.1.2 Case - Yangtze River flood modeling

Yangtze River in China is one largest in the world and some 400 million people live in the area watered by it. There are lots of dams and hydroelectric power plants along the Yangtze. The flow data has been generated for six rivers and for a period of 50 years.

The decision-making situation has two contradicting goals. For maximal productivity of a power plant, the surface of a basin should be as high as possible. The safest situation conversely is that the surface is as low as possible.

1.1.3 Assignment and objectives

EIA formulated the following tasks to the project group:

1. Determine the probability that certain river flows (case Yangtze) exceed maximum tolerance given in 50, 100 and 200 years, provided that long history of the flows is given.
2. Determine optimal actions concerning basin and reserve area usage at every moment with the condition that the risk level is to be less than 0.001. At the starting point all dam and river areas are at their minimum level. Future flows are not known at the decision point.
3. Design a dynamic method or application to produce optimal actions, when costs and risk levels are known.
4. Apply this method to case Yangtze.

EIA delivered us flow data of 6 tributary river of Yangtze, measured daily over 50 years. Also the cost levels of floods, capacity of basins and the delays of fulfillments of the reserve areas were known. The main goal in the project was the design of the decision-making method (mentioned above in 3. task) to aid in taking optimal actions.

1.1.4 Structure of the project

The project can be seen to have consisted of two main parts. The first part was to analyze the flow data given. The first idea was to identify the SARIMA-processes of flows and to estimate the parameters. This would probably be the most suitable approach, but because the assignment was also to calculate probabilities for long term events, we decided not to use time series analysis.

The bigger or at least more relevant part of this project was to analyze the decision-making situation. The goal was to come up with some method, which determines what actions to take under certain conditions.

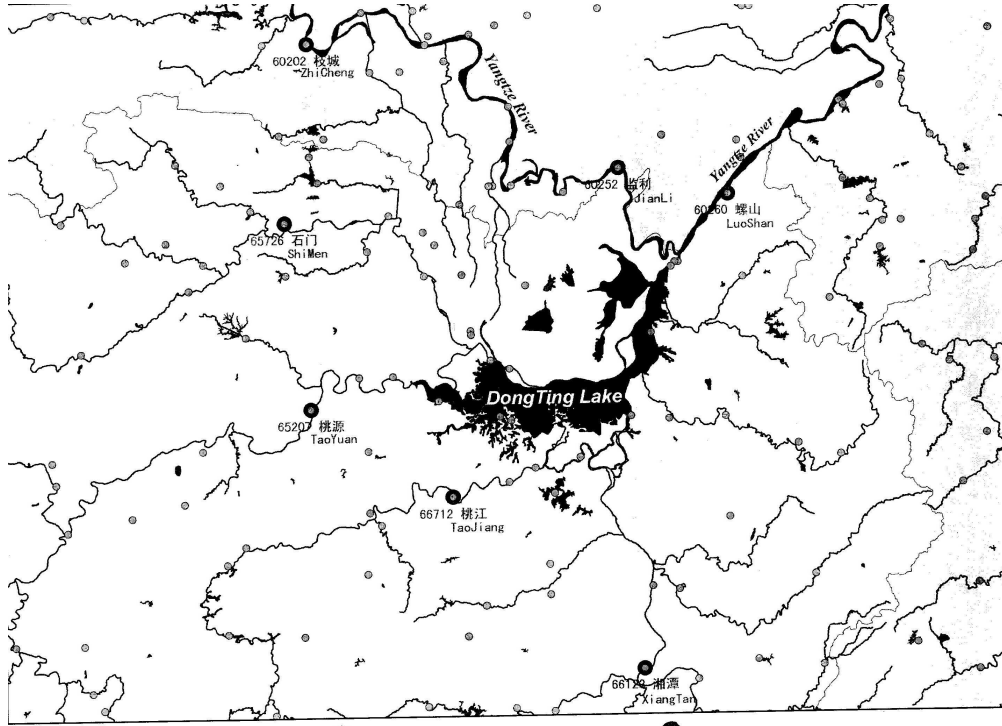


Figure 1: The Yangtze river and the surrounding areas.

1.1.5 Project execution

The greatest problem in achieving the objectives of the project was that the group worked on a virtual basis. Each member lived in different town and the group communicated mainly through e-mail. This hindered the effectiveness of communication, reduced the exchange of ideas and complicated the execution of the project.

Despite the careful planning of actions in the project the risks in many ways came true. Virtual meetings between team members did not compensate the need for real meetings which all could take place. This hindered in advancing in the project. Afterwards, also the management of the team could have been more dispersed to all the team members.

Since there was only one person really responsible of designing the method to be used in the project case, this would have facilitated the project as well. Much time was also used for clarifying the assignment as well. This could have been easily bypassed by more accurate project information in the beginning of project from EIA.

The co-operation it self worked out fine, both to EIA and within the team. Team spirit was good and open for discussions to solve problems of all kinds. However, there is still much learning to do in project work. Especially the schedule must be carefully followed to able to attain results within the time limits.

1.2 Literature review

The project group carried out a literary review to be aware of the theoretical background of the assignment. When the project was initiated, no one in the project group was familiar with flood modeling or environmental risk analysis. The group discovered that the amount of applicable previous studies on the subject was to some extent limited. Therefore the process of designing the decision-making system was quite intuitive in nature. The most relevant piece of research for designing the decision-making system is presented below.

These studies with different mathematical methods have been done based on different kinds of time series. One aspect is to measure daily rainfall in certain area. More common approach ist to monitor the streamflow, as W. Boughton et al [4] has demonstrated.

Analysing rainfall time series , or any other data than flows, requires a lot more complex analysis, since rainfall and river flows can be rather uncorrelated. In this light, the use of river flow data has a straightforward basis.

Zhang et al (2002) presented an application of an improved linear storage

routing model for the estimation of large floods. They emphasized that while conceptual storage routing models have been developed for the same purpose, one of their key components had been the assumed non-linear storage-discharge relationship. In addition, Australian Rainfall and Runoff (Institution of Engineers, 1987) had actually recommended this relationship to be used in the estimation of large floods.

However, also models with other kinds of storage-discharge relationships have been developed. The relationship has been modeled e.g. as asymptotically linear, non-linear with an intercept and as linear. As to which relationship best models the phenomenon, it has been shown that catchments tend to operate as linear systems in larger floods (Zhang, 2001).

In addition, the project group studied articles and literature on flood forecasting (Toth et al., 1998; Strupczewski et al., 2000) and environmental risk analysis in general (Lerche & Paleologos, 2001).

The theoretical approach of this study shared a similar line of thought with the work of Zhang. The theoretical approach will be presented in more detail in subsequent chapters.

1.3 Structure of the report

After this introductory chapter, the decision-making situation will be described in the next chapter. In the third chapter, our approach will be presented in more detail. The chapter also covers the restrictions of our solution and the formulation of the problem. The fourth chapter consists of a discussion of the contribution of the decision-making system. Finally, in the fifth chapter the decision-making system is applied to the case of Yangtze River.

2 Decision situation

The decision situation of systems with many tributary rivers can be divided into parts and each river can be treated independently. In the case of tributary rivers the net effect can be modeled by analysing the sum flows. Thus, every system can be divided into smaller subproblems which can be analyzed as a one basin systems. In future we will concentrate in these so called “one-basin systems”.

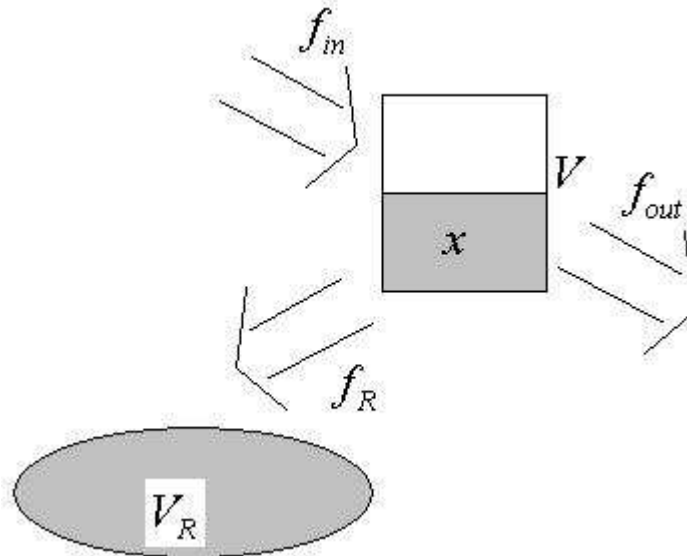


Figure 2: Schematic diagram of a one-basin system. In the middle there is the basin and in the lower left corner the reservoir area.

Assume the basin volume to be V . In the decision situation the current water level of the basin is known. The electricity produced depends linearly on the water level. Assume that the gain of maximum electricity production to be M . There are also an option of using the reservoir area. This involves a non-recurring cost of C . Assume that the volume of the reservoir area is V_R and it fills with constant speed until time T_R (then it's full). If the basin floods, there will be a fine (penalty) of F . The flow put into the river from the basin must not exceed K .

3 Our approach

The data given by EIA of daily measurements made of flow rates from main and tributary rivers from last 50 years leads to use the statistical means in analysing the flood risks. Generally, flow data is typically collected in some form, so our approach presented below is fairly applicable.

In analyzing the river floods the main interest is to determine the floods probabilities corresponding the decisions made about the basin surface height. We think it is natural to assume that some kind of distributions of the river flows can be estimated. This is also a natural way of approaching the probabilities mentioned above.

This probabilistic approach is easily applied to different kinds of systems, but this generality of course requires simplification of systems.

We approach the decision-making in a following sense: decisions are made discretely at (constant) Δt time intervals. Δt should be chosen small enough, so that this discretation isn't too robust. If $\Delta t \rightarrow 0$ the decision-making is in a sense continuous.

3.1 Problem formulation

Naturally the optimization task is to maximize the financial profit of the hydroelectric power plant. If the reservoir usage cost C and flood penalty F are rather small the optimal solution would result from keeping the water level as high as possible throughout the year and accept either the reservoir usage cost C or the flood fine F . To be able to take into account also the humane risks, the risk level itself should also be controlled in some way.

Thus, for humane and political causes it is necessary to add a constraint to keep the risk level low enough. We use a very intuitive constraint, the risk level of the flood must stay below 0.001. Probability $P(\text{flood isn't avoidable})$ is quite hard to calculate, because it consists of the events that flood isn't avoidable and occurs at latest at time $t_i, i = 1 \dots$

This problem can be set aside, because the probabilities that the flood occurs much later than now are very small. Thus, the risk level can be approximated by discarding these probabilities.

Denote the degree of fullness of the basin at the time t_n by x_n . Formulating our approach into an optimization problem, at time t_n the decision problem is

$$\begin{aligned}
& \max M * \min(1, E[x_{n+1}]) - (1 - y_{1,n-1}) * y_{1,n} * C - y_{2,n} * F \\
& \text{s.t. } P(x_{n+1} > 1 \text{ or } x_{n+2} > 1) \leq 0.001
\end{aligned} \tag{1}$$

where,

$$x_{n+1} = x_n + (V_{in,n} - V_{out,n})/V, x_n \geq 0$$

$V_{in,n}$ is the volume of the water flow into the basin from the river.

$V_{in,n}$ is a random variable and thus x_{n+1} is also a random variable.

$V_{out,n}$ is the volume of the water passed through the dam or put into reservoir area. $V_{out,n}$ is a direct consequence of our decisions.

$y_{1,n}$ is the indicator of using the reservoir area, and a decision variable

$y_{2,n}$ is the indicator of flood.

$y_{1,n}$, $y_{2,n}$ and $V_{out,n}$ can be expressed in

$$\begin{aligned}
y_{1,n} &= \begin{cases} 1, & \text{if reservoir area is used} \\ 0, & \text{otherwise} \end{cases} \\
y_{2,n} &= \begin{cases} 1, & \text{if } x_{n+1} > 1 \\ 0, & \text{otherwise} \end{cases} \\
V_{out,n} &= f_{out,n} + y_{1,n} * \frac{V_R}{T_R}
\end{aligned}$$

and $f_{out,n}$ is of course a decision variable, $0 \leq f_{out,n} \leq K$.

3.2 Restrictions

The most obvious restriction of this approach is that some kind of estimated distribution of future river flows are assumed to be available. As already mentioned earlier, this data is although often collected and this restriction isn't at all limiting.

The decision is made discretely and this might be a restriction to some dynamic models. Although the qualitative behaviour of the model should be sufficiently dynamic, when Δt is chosen small enough.

All in all, this approach is in its plainness quite flexible and very applicable to different situations because the main idea is very general.

4 Discussion

This method that has been generated is very straightforward and applicable. There are certain weaknesses or disadvantages concerning the model, and these will be analysed in this section.

4.1 The dynamics of the model

When collecting the data it is important to be sure that the measurements will give an representative sample of the flow or water level behaviour. If the water level is fluctuating all the time, it is possible that the flow maximum will occur between the measurements.

For example if the data is measured daily at noon, but the flow fluctuates throughout the day, the measurements won't give a proper view of the situation. It is also possible that the data isn't representative in the sense that it doesn't show out longer-term effects, e.g. greenhouse effect.

Choosing a good time interval for measurements requires knowledge of the qualitative behaviour of the flood. This time interval is also very closely related to the Δt in the decision-making method. Both intervals should be chosen so that the flows wouldn't fluctuate too much during this interval. It would naturally be suitable to choose this measurement and decision time interval to same Δt .

Analytically it is hard to determine, how small Δt should be for the decision-making to "work". Although, as will be seen later in the case test runs, the

river flows maybe quite static and also a big Δt can handle the situation. In the this case example the values for $\Delta t = 1$ day and $\Delta t = 2$ days were tested. When determining a suitable time interval, one should always think how rapidly the river flows change and choose a value for Δt accordingly.

4.2 Estimating the flow distributions

In the method the most binding assumption is that the future flow distributions can be estimated. Even if the estimating can be done, there is uncertainty in the estimates. For example, if the distribution is parametric, the estimated parameter(s) can be biased. This problem dealing with the estimation biases isn't analyzed in this paper, nut being aware of this risk factor can be crucial.

4.3 Calculating the risk levels

The risk level for flood to occur is very hard to calculate, because the inevitable flood can theoretically realize also after a very long period of time. To ease the calculation these probabilities can be cut off and the risk at time t_n can be thought to consist of the events ' The flood occurs at time t_k ', $n \leq k \leq n + m$

These "cut-offs" ease the calculation rapidly, but the disadvantages aren't that big at all. These "tail probabilities" are normally very small.

5 Case Yangtze

5.1 Data analysis

The data was classified both to daily (to 365 cases) and to montly classes. Little surprisingly, none the samples could be identified to be produced by a normal distribution. Instead, the histograms refered to χ^2 -distributions. This

interesting fact is though as such irrelevant to this decision-making method and no statistical testing was made for this hypothesis. The statistics of sum flow and flow2 are represented in appendices.

Let denote the events

A_i ='SUMFLOW doesn't exceed 78500 m^3/s in month i ' and

B_i ='FLOW2 doesn't exceed 5500 m^3/s in month i .' $i = 1, \dots, 12$

and $P(A_i) =: p_i$ and $P(B_i) = q_i$. These probabilities can be estimated by histogram method (presented e.g. by Clemen [5]). For example

$$p_1 = 1, q_7 = \left(\frac{1538}{1550}\right)^{31} = 0.78589, p_6 = \left(\frac{1491}{1500}\right)^{30} = 0.83482$$

Now, probabilities

$P(\text{SUMFLOW doesn't exceed } 78500 \text{ } m^3/s \text{ in } N \text{ years})$

$$= \left(\prod_{i=1}^{12} p_i^N\right) = (0.88671 * 0.88671 * 0.69621)^N$$

$P(\text{FLOW2 doesn't exceed } 5500 \text{ } m^3/s \text{ in } N \text{ years})$

$$= \left(\prod_{i=1}^{12} q_i^N\right) = (0.88671 * 0.94171 * 0.80189)^N$$

Both probabilities are smaller than 10^{-8} , when $N \geq 50$. The fact that these probabilities are very very small isn't that surprising, because we are dealing with very long periods of time. It would, indeed be unexpected that during a 50 year period, there were no floods.

This approach has although a remarkable theoretical weakness: it doesn't take into account dependencies between daily flows. One could for example imagine that floods last for at least some days. In general, assumption of independent daily flows is hardly true and better approach would be to examine the monthly maximum flows.

This kind of aspect wouldn't though be applicable for the decision-making method constructed in chapter 3.1. We also supposed that the error caused by this assumption is reasonable and on the other hand it allows the usage of the decision-making method.

5.2 Method demonstration

Now let's demonstrate the decision-making method. The method is applied to one of the tributary rivers of Yangtze. The data is given by EIA and the date is July 10th, 1984. The parameters are as follows:

The basin volume	$V = 1.5 * 10^9 m^3$
The gain from electricity production	$M = 10^6 \text{Eur}$
Volume of the reservoir area	$V_R = 0.3 * 10^9 m^3$
Cost from using the reservoir area	$C = 50 * 10^6 \text{Eur}$
The time it takes to full the reservoir area	$T_R = 3 \text{ days}$
The fine from flood	$F = 500 * 10^6 \text{Eur}$
The maximum flow in the river	$K = 5500 m^3/s$

Since the data has been given in daily intervals, logical idea is to choose the interval also for the model to be the same, i.e. $\Delta t = 1$ day. Choosing a shorted period would lead to new problemes and it would require further assumptions of the water level change and also the generation of the data for these moments as well. For the sake of researching the effect of Δt same situation is demonstrated with a run of $\Delta t = 2$ days.

At each decision state the problem (1) is solved. It is important to notice that in calculating the risk level, the approximated density function of the $f_{in,n+1}$ is utilized.

The results are rather good and both values for Δt lead to quite similar behaviour. The run with $\Delta t = 2$ days is probably a bit more careful. This is also very intuitive, because the variance of $f_{in,n+1}$ is four times as big as with $\Delta t = 1$ day, while the expected value is just twice as big.

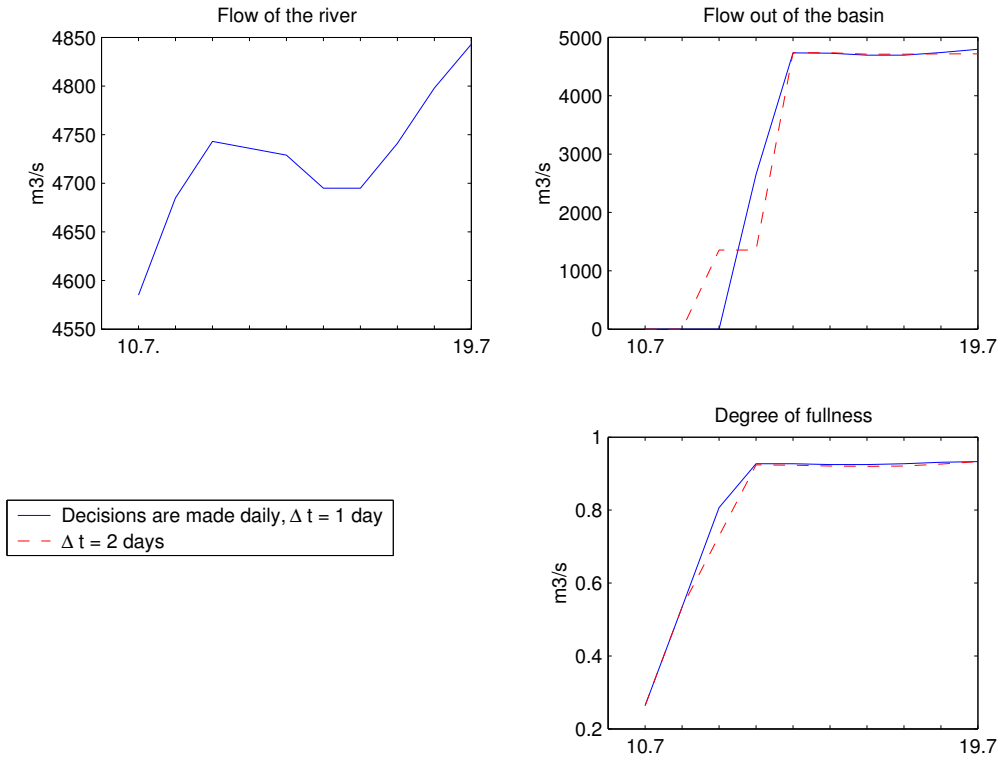


Figure 3: Results of a test run

In both runs the degree of fullness of the basin first grow from 0 to over 0.9 and quickly stabilizes little above 0.9. This method seems to handle this situation fairly well.

References

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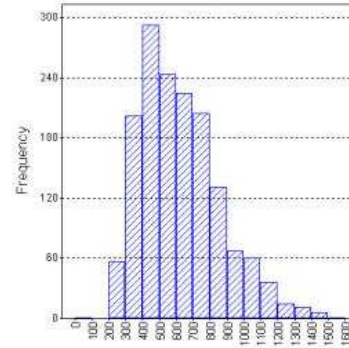
Appendices

Flow 2 and SUMFLOW data sorted by month.

STATISTIX FOR WINDOWS
FREQUENCY DISTRIBUTION OF FLOW2

LOW	HIGH	FREQ	PERCENT	CUMULATIVE FREQ	CUMULATIVE PERCENT
0	100	1	0.1	1	0.1
100	200	0	0.0	1	0.1
200	300	57	3.7	58	3.7
300	400	202	13.0	260	16.8
400	500	292	18.8	552	35.6
500	600	243	15.7	795	51.3
600	700	224	14.5	1019	65.7
700	800	204	13.2	1223	78.9
800	900	131	8.5	1354	87.4
900	1000	67	4.3	1421	91.7
1000	1100	60	3.9	1481	95.5
1100	1200	36	2.3	1517	97.9
1200	1300	15	1.0	1532	98.8
1300	1400	11	0.7	1543	99.5
1400	1500	6	0.4	1549	99.9
1500	1600	1	0.1	1550	100.0
TOTAL		1550	100.0		

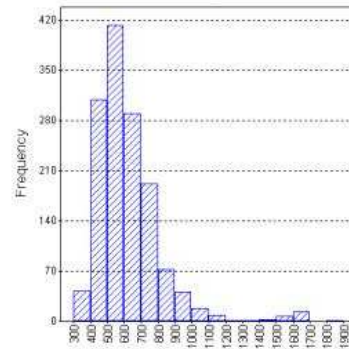
January



STATISTIX FOR WINDOWS
FREQUENCY DISTRIBUTION OF FLOW2

LOW	HIGH	FREQ	PERCENT	CUMULATIVE FREQ	CUMULATIVE PERCENT
300	400	42	3.0	42	3.0
400	500	309	21.9	351	24.9
500	600	413	29.2	764	54.1
600	700	290	20.5	1054	74.6
700	800	192	13.6	1246	88.2
800	900	73	5.2	1319	93.4
900	1000	41	2.9	1360	96.3
1000	1100	18	1.3	1378	97.6
1100	1200	9	0.6	1387	98.2
1200	1300	1	0.1	1388	98.3
1300	1400	1	0.1	1389	98.4
1400	1500	2	0.1	1391	98.5
1500	1600	7	0.5	1398	99.0
1600	1700	13	0.9	1411	99.9
1700	1800	0	0.0	1411	99.9
1800	1900	1	0.1	1412	100.0
TOTAL		1412	100.0		

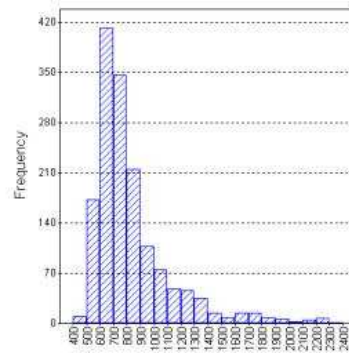
February



STATISTIX FOR WINDOWS
FREQUENCY DISTRIBUTION OF FLOW2

LOW	HIGH	FREQ	PERCENT	CUMULATIVE FREQ	CUMULATIVE PERCENT
400	500	10	0.6	10	0.6
500	600	172	11.1	182	11.7
600	700	412	26.6	594	38.3
700	800	346	22.3	940	60.6
800	900	215	13.9	1155	74.5
900	1000	108	7.0	1263	81.5
1000	1100	75	4.8	1338	86.3
1100	1200	48	3.1	1386	89.4
1200	1300	46	3.0	1432	92.4
1300	1400	35	2.3	1467	94.6
1400	1500	15	1.0	1482	95.6
1500	1600	8	0.5	1490	96.1
1600	1700	14	0.9	1504	97.0
1700	1800	15	1.0	1519	98.0
1800	1900	9	0.6	1528	98.6
1900	2000	6	0.4	1534	99.0
2000	2100	3	0.2	1537	99.2
2100	2200	5	0.3	1542	99.5
2200	2300	7	0.5	1549	99.9
2300	2400	1	0.1	1550	100.0
TOTAL		1550	100.0		

March

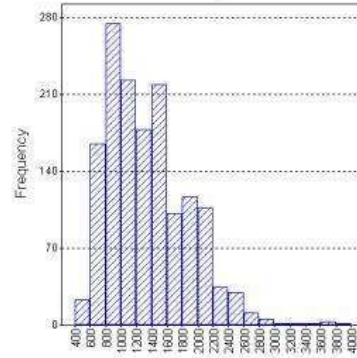


STATISTIX FOR WINDOWS

FREQUENCY DISTRIBUTION OF FLOW2

April

LOW	HIGH	FREQ	PERCENT	CUMULATIVE FREQ	CUMULATIVE PERCENT
400	600	23	1.5	23	1.5
600	800	165	11.0	188	12.5
800	1000	274	18.3	462	30.8
1000	1200	223	14.9	685	45.7
1200	1400	178	11.9	863	57.5
1400	1600	219	14.6	1082	72.1
1600	1800	101	6.7	1183	78.9
1800	2000	117	7.8	1300	86.7
2000	2200	107	7.1	1407	93.8
2200	2400	35	2.3	1442	96.1
2400	2600	30	2.0	1472	98.1
2600	2800	11	0.7	1483	98.9
2800	3000	6	0.4	1489	99.3
3000	3200	2	0.1	1491	99.4
3200	3400	2	0.1	1493	99.5
3400	3600	2	0.1	1495	99.7
3600	3800	3	0.2	1498	99.9
3800	4000	2	0.1	1500	100.0
TOTAL		1500	100.0		

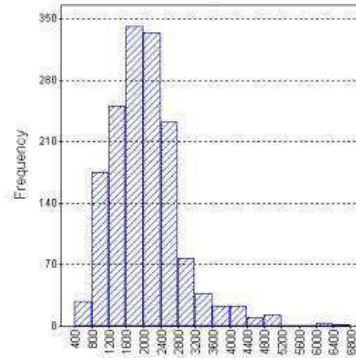


STATISTIX FOR WINDOWS

FREQUENCY DISTRIBUTION OF FLOW2

May

LOW	HIGH	FREQ	PERCENT	CUMULATIVE FREQ	CUMULATIVE PERCENT
400	800	28	1.8	28	1.8
800	1200	175	11.3	203	13.1
1200	1600	250	16.1	453	29.2
1600	2000	341	22.0	794	51.2
2000	2400	334	21.5	1128	72.8
2400	2800	232	15.0	1360	87.7
2800	3200	77	5.0	1437	92.7
3200	3600	37	2.4	1474	95.1
3600	4000	23	1.5	1497	96.6
4000	4400	23	1.5	1520	98.1
4400	4800	10	0.6	1530	98.7
4800	5200	13	0.8	1543	99.5
5200	5600	1	0.1	1544	99.6
5600	6000	1	0.1	1545	99.7
6000	6400	3	0.2	1548	99.9
6400	6800	2	0.1	1550	100.0
TOTAL		1550	100.0		

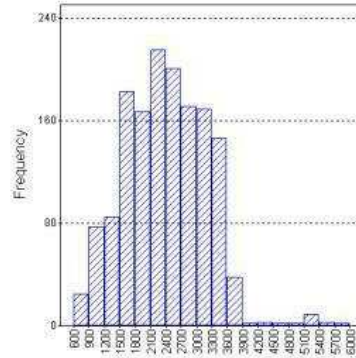


STATISTIX FOR WINDOWS

FREQUENCY DISTRIBUTION OF FLOW2

June

LOW	HIGH	FREQ	PERCENT	CUMULATIVE FREQ	CUMULATIVE PERCENT
600	900	25	1.7	25	1.7
900	1200	77	5.1	102	6.8
1200	1500	85	5.7	187	12.5
1500	1800	183	12.2	370	24.7
1800	2100	167	11.1	537	35.8
2100	2400	215	14.3	752	50.1
2400	2700	201	13.4	953	63.5
2700	3000	171	11.4	1124	74.9
3000	3300	169	11.3	1293	86.2
3300	3600	146	9.7	1439	95.9
3600	3900	38	2.5	1477	98.5
3900	4200	2	0.1	1479	98.6
4200	4500	3	0.2	1482	98.8
4500	4800	2	0.1	1484	98.9
4800	5100	2	0.1	1486	99.1
5100	5400	9	0.6	1495	99.7
5400	5700	3	0.2	1498	99.9
5700	6000	2	0.1	1500	100.0
TOTAL		1500	100.0		



July

STATISTIX FOR WINDOWS

FREQUENCY DISTRIBUTION OF FLOW2

LOW	HIGH	FREQ	PERCENT	CUMULATIVE FREQ	CUMULATIVE PERCENT
600	900	8	0.5	8	0.5
900	1200	6	0.4	14	0.9
1200	1500	44	2.8	58	3.7
1500	1800	91	5.9	149	9.6
1800	2100	110	7.1	259	16.7
2100	2400	192	12.4	451	29.1
2400	2700	239	15.4	690	44.5
2700	3000	198	12.8	888	57.3
3000	3300	148	9.5	1036	66.8
3300	3600	161	10.4	1197	77.2
3600	3900	96	6.2	1293	83.4
3900	4200	87	5.6	1380	89.0
4200	4500	63	4.1	1443	93.1
4500	4800	45	2.9	1488	96.0
4800	5100	23	1.5	1511	97.5
5100	5400	22	1.4	1533	98.9
5400	5700	13	0.8	1546	99.7
5700	6000	4	0.3	1550	100.0
TOTAL		1550	100.0		

STATISTIX FOR WINDOWS **August**

FREQUENCY DISTRIBUTION OF FLOW2

LOW	HIGH	FREQ	PERCENT	CUMULATIVE FREQ	CUMULATIVE PERCENT
600	800	3	0.2	3	0.2
800	1000	3	0.2	6	0.4
1000	1200	3	0.2	9	0.6
1200	1400	20	1.3	29	1.9
1400	1600	64	4.1	93	6.0
1600	1800	82	5.3	175	11.3
1800	2000	110	7.1	285	18.4
2000	2200	164	10.6	449	29.0
2200	2400	159	10.3	608	39.2
2400	2600	174	11.2	782	50.5
2600	2800	188	12.1	970	62.6
2800	3000	175	11.3	1145	73.9
3000	3200	115	7.4	1260	81.3
3200	3400	92	5.9	1352	87.2
3400	3600	72	4.6	1424	91.9
3600	3800	27	1.7	1451	93.6
3800	4000	32	2.1	1483	95.7
4000	4200	25	1.6	1508	97.3
4200	4400	18	1.2	1526	98.5
4400	4600	10	0.6	1536	99.1
4600	4800	13	0.8	1549	99.9
4800	5000	0	0.0	1549	99.9
5000	5200	1	0.1	1550	100.0
TOTAL		1550	100.0		

September

STATISTIX FOR WINDOWS

FREQUENCY DISTRIBUTION OF FLOW2

LOW	HIGH	FREQ	PERCENT	CUMULATIVE FREQ	CUMULATIVE PERCENT
1000	1200	8	0.5	8	0.5
1200	1400	16	1.1	24	1.6
1400	1600	37	2.5	61	4.1
1600	1800	69	4.6	130	8.7
1800	2000	117	7.8	247	16.5
2000	2200	153	10.2	400	26.7
2200	2400	233	15.5	633	42.2
2400	2600	253	16.9	886	59.1
2600	2800	198	13.2	1084	72.3
2800	3000	85	5.7	1169	77.9
3000	3200	94	6.3	1263	84.2
3200	3400	77	5.1	1340	89.3
3400	3600	61	4.1	1401	93.4
3600	3800	30	2.0	1431	95.4
3800	4000	48	3.2	1479	98.6
4000	4200	17	1.1	1496	99.7
4200	4400	4	0.3	1500	100.0
TOTAL		1500	100.0		

October

STATISTIX FOR WINDOWS

FREQUENCY DISTRIBUTION OF FLOW2

LOW	HIGH	FREQ	PERCENT	CUMULATIVE FREQ	CUMULATIVE PERCENT
400	600	1	0.1	1	0.1
600	800	1	0.1	2	0.1
800	1000	17	1.1	19	1.2
1000	1200	5	0.3	24	1.5
1200	1400	40	2.6	64	4.1
1400	1600	159	10.3	223	14.4
1600	1800	154	9.9	377	24.3
1800	2000	198	12.8	575	37.1
2000	2200	290	18.7	865	55.8
2200	2400	174	11.2	1039	67.0
2400	2600	179	11.5	1218	78.6
2600	2800	149	9.6	1367	88.2
2800	3000	88	5.7	1455	93.9
3000	3200	66	4.3	1521	98.1
3200	3400	28	1.8	1549	99.9
3400	3600	1	0.1	1550	100.0
TOTAL		1550	100.0		

November

STATISTIX FOR WINDOWS

FREQUENCY DISTRIBUTION OF FLOW2

LOW	HIGH	FREQ	PERCENT	CUMULATIVE FREQ	CUMULATIVE PERCENT
400	500	7	0.5	7	0.5
500	600	46	3.1	53	3.5
600	700	46	3.1	99	6.6
700	800	43	2.9	142	9.5
800	900	56	3.7	198	13.2
900	1000	102	6.8	300	20.0
1000	1100	157	10.5	457	30.5
1100	1200	125	8.3	582	38.8
1200	1300	87	5.8	669	44.6
1300	1400	90	6.0	759	50.6
1400	1500	99	6.6	858	57.2
1500	1600	152	10.1	1010	67.3
1600	1700	96	6.4	1106	73.7
1700	1800	90	6.0	1196	79.7
1800	1900	57	3.8	1253	83.5
1900	2000	47	3.1	1300	86.7
2000	2100	39	2.6	1339	89.3
2100	2200	47	3.1	1386	92.4
2200	2300	48	3.2	1434	95.6
2300	2400	40	2.7	1474	98.3
2400	2500	10	0.7	1484	98.9
2500	2600	15	1.0	1499	99.9
2600	2700	1	0.1	1500	100.0
TOTAL		1500	100.0		

December

STATISTIX FOR WINDOWS

FREQUENCY DISTRIBUTION OF FLOW2

LOW	HIGH	FREQ	PERCENT	CUMULATIVE FREQ	CUMULATIVE PERCENT
300	400	22	1.4	22	1.4
400	500	83	5.4	105	6.8
500	600	94	6.1	199	12.8
600	700	152	9.8	351	22.6
700	800	156	10.1	507	32.7
800	900	228	14.7	735	47.4
900	1000	212	13.7	947	61.1
1000	1100	188	12.1	1135	73.2
1100	1200	116	7.5	1251	80.7
1200	1300	89	5.7	1340	86.5
1300	1400	44	2.8	1384	89.3
1400	1500	46	3.0	1430	92.3
1500	1600	41	2.6	1471	94.9
1600	1700	32	2.1	1503	97.0
1700	1800	15	1.0	1518	97.9
1800	1900	16	1.0	1534	99.0
1900	2000	13	0.8	1547	99.8
2000	2100	3	0.2	1550	100.0
TOTAL		1550	100.0		

STATISTIX FOR WINDOWS **January**
 FREQUENCY DISTRIBUTION OF SUMFLOW

LOW	HIGH	FREQ	PERCENT	CUMULATIVE	
				FREQ	PERCENT
4000	5000	10	0.6	10	0.6
5000	6000	124	8.0	134	8.6
6000	7000	149	9.6	283	18.3
7000	8000	177	11.4	460	29.7
8000	9000	220	14.2	680	43.9
9000	10000	203	13.1	883	57.0
10000	11000	200	12.9	1083	69.9
11000	12000	158	10.2	1241	80.1
12000	13000	87	5.6	1328	85.7
13000	14000	47	3.0	1375	88.7
14000	15000	45	2.9	1420	91.6
15000	16000	55	3.5	1475	95.2
16000	17000	31	2.0	1506	97.2
17000	18000	16	1.0	1522	98.2
18000	19000	6	0.4	1528	98.6
19000	20000	5	0.3	1533	98.9
20000	21000	5	0.3	1538	99.2
21000	22000	5	0.3	1543	99.5
22000	23000	3	0.2	1546	99.7
23000	24000	2	0.1	1548	99.9
24000	25000	0	0.0	1548	99.9
25000	26000	1	0.1	1549	99.9
26000	27000	0	0.0	1549	99.9
27000	28000	1	0.1	1550	100.0
TOTAL		1550	100.0		

February
 STATISTIX FOR SUMFLOW
 FREQUENCY DISTRIBUTION OF V003

LOW	HIGH	FREQ	PERCENT	CUMULATIVE	
				FREQ	PERCENT
5000	6000	3	0.2	3	0.2
6000	7000	75	5.3	78	5.5
7000	8000	211	14.9	289	20.5
8000	9000	357	25.3	646	45.8
9000	10000	240	17.0	886	62.7
10000	11000	172	12.2	1058	74.9
11000	12000	111	7.9	1169	82.8
12000	13000	64	4.5	1233	87.3
13000	14000	55	3.9	1288	91.2
14000	15000	34	2.4	1322	93.6
15000	16000	23	1.6	1345	95.3
16000	17000	18	1.3	1363	96.5
17000	18000	8	0.6	1371	97.1
18000	19000	14	1.0	1385	98.1
19000	20000	17	1.2	1402	99.3
20000	21000	8	0.6	1410	99.9
21000	22000	1	0.1	1411	99.9
22000	23000	1	0.1	1412	100.0
TOTAL		1412	100.0		

STATISTIX FOR WINDOWS **March**
 FREQUENCY DISTRIBUTION OF SUMFLOW

LOW	HIGH	FREQ	PERCENT	CUMULATIVE	
				FREQ	PERCENT
6000	7000	3	0.2	3	0.2
7000	8000	8	0.5	11	0.7
8000	9000	21	1.4	32	2.1
9000	10000	127	8.2	159	10.3
10000	11000	311	20.1	470	30.3
11000	12000	259	16.7	729	47.0
12000	13000	265	17.1	994	64.1
13000	14000	168	10.8	1162	75.0
14000	15000	85	5.5	1247	80.5
15000	16000	62	4.0	1309	84.5
16000	17000	39	2.5	1348	87.0
17000	18000	24	1.5	1372	88.5
18000	19000	26	1.7	1398	90.2
19000	20000	34	2.2	1432	92.4
20000	21000	25	1.6	1457	94.0
21000	22000	24	1.5	1481	95.5
22000	23000	14	0.9	1495	96.5
23000	24000	15	1.0	1510	97.4
24000	25000	13	0.8	1523	98.3
25000	26000	8	0.5	1531	98.8
26000	27000	13	0.8	1544	99.6
27000	28000	2	0.1	1546	99.7
28000	29000	3	0.2	1549	99.9
29000	30000	1	0.1	1550	100.0
TOTAL		1550	100.0		

STATISTIX FOR WINDOWS **April**
 FREQUENCY DISTRIBUTION OF SUMFLOW

LOW	HIGH	FREQ	PERCENT	CUMULATIVE	
				FREQ	PERCENT
8000	10000	38	2.5	38	2.5
10000	12000	60	4.0	98	6.5
12000	14000	155	10.3	253	16.9
14000	16000	171	11.4	424	28.3
16000	18000	159	10.6	583	38.9
18000	20000	137	9.1	720	48.0
20000	22000	155	10.3	875	58.3
22000	24000	115	7.7	990	66.0
24000	26000	96	6.4	1086	72.4
26000	28000	97	6.5	1183	78.9
28000	30000	72	4.8	1255	83.7
30000	32000	75	5.0	1330	88.7
32000	34000	56	3.7	1386	92.4
34000	36000	21	1.4	1407	93.8
36000	38000	43	2.9	1450	96.7
38000	40000	21	1.4	1471	98.1
40000	42000	10	0.7	1481	98.7
42000	44000	6	0.4	1487	99.1
44000	46000	4	0.3	1491	99.4
46000	48000	3	0.2	1494	99.6
48000	50000	4	0.3	1498	99.9
50000	52000	1	0.1	1499	99.9
52000	54000	0	0.0	1499	99.9
54000	56000	1	0.1	1500	100.0
TOTAL		1500	100.0		

STATISTIX FOR WINDOWS **May**
 FREQUENCY DISTRIBUTION OF SUMFLOW

LOW	HIGH	FREQ	PERCENT	CUMULATIVE	
				FREQ	PERCENT
8000	12000	27	1.7	27	1.7
12000	16000	70	4.5	97	6.3
16000	20000	93	6.0	190	12.3
20000	24000	151	9.7	341	22.0
24000	28000	227	14.6	568	36.6
28000	32000	243	15.7	811	52.3
32000	36000	242	15.6	1053	67.9
36000	40000	231	14.9	1284	82.8
40000	44000	106	6.8	1390	89.7
44000	48000	45	2.9	1435	92.6
48000	52000	32	2.1	1467	94.6
52000	56000	11	0.7	1478	95.4
56000	60000	27	1.7	1505	97.1
60000	64000	17	1.1	1522	98.2
64000	68000	13	0.8	1535	99.0
68000	72000	2	0.1	1537	99.2
72000	76000	4	0.3	1541	99.4
76000	80000	5	0.3	1546	99.7
80000	84000	4	0.3	1550	100.0
TOTAL		1550	100.0		

STATISTIX FOR WINDOWS **June**
 FREQUENCY DISTRIBUTION OF SUMFLOW

LOW	HIGH	FREQ	PERCENT	CUMULATIVE	
				FREQ	PERCENT
8000	12000	6	0.4	6	0.4
12000	16000	24	1.6	30	2.0
16000	20000	23	1.5	53	3.5
20000	24000	89	5.9	142	9.5
24000	28000	123	8.2	265	17.7
28000	32000	171	11.4	436	29.1
32000	36000	207	13.8	643	42.9
36000	40000	171	11.4	814	54.3
40000	44000	136	9.1	950	63.3
44000	48000	228	15.2	1178	78.5
48000	52000	173	11.5	1351	90.1
52000	56000	49	3.3	1400	93.3
56000	60000	49	3.3	1449	96.6
60000	64000	27	1.8	1476	98.4
64000	68000	9	0.6	1485	99.0
68000	72000	5	0.3	1490	99.3
72000	76000	1	0.1	1491	99.4
76000	80000	4	0.3	1495	99.7
80000	84000	5	0.3	1500	100.0
TOTAL		1500	100.0		

STATISTIX FOR SUMFLOW **July**
 FREQUENCY DISTRIBUTION OF SUMFLOW

LOW	HIGH	FREQ	PERCENT	CUMULATIVE	
				FREQ	PERCENT
8000	12000	8	0.5	8	0.5
12000	16000	4	0.3	12	0.8
16000	20000	4	0.3	16	1.0
20000	24000	17	1.1	33	2.1
24000	28000	44	2.8	77	5.0
28000	32000	100	6.5	177	11.4
32000	36000	143	9.2	320	20.6
36000	40000	144	9.3	464	29.9
40000	44000	175	11.3	639	41.2
44000	48000	202	13.0	841	54.3
48000	52000	163	10.5	1004	64.8
52000	56000	173	11.2	1177	75.9
56000	60000	145	9.4	1322	85.3
60000	64000	72	4.6	1394	89.9
64000	68000	76	4.9	1470	94.8
68000	72000	40	2.6	1510	97.4
72000	76000	16	1.0	1526	98.5
76000	80000	11	0.7	1537	99.2
80000	84000	13	0.8	1550	100.0
TOTAL		1550	100.0		

STATISTIX FOR WINDOWS **September**
 FREQUENCY DISTRIBUTION OF SUMFLOW

LOW	HIGH	FREQ	PERCENT	CUMULATIVE	
				FREQ	PERCENT
20000	22000	5	0.3	5	0.3
22000	24000	15	1.0	20	1.3
24000	26000	25	1.7	45	3.0
26000	28000	37	2.5	82	5.5
28000	30000	68	4.5	150	10.0
30000	32000	68	4.5	218	14.5
32000	34000	106	7.1	324	21.6
34000	36000	154	10.3	478	31.9
36000	38000	142	9.5	620	41.3
38000	40000	136	9.1	756	50.4
40000	42000	185	12.3	941	62.7
42000	44000	129	8.6	1070	71.3
44000	46000	98	6.5	1168	77.9
46000	48000	108	7.2	1276	85.1
48000	50000	65	4.3	1341	89.4
50000	52000	26	1.7	1367	91.1
52000	54000	15	1.0	1382	92.1
54000	56000	27	1.8	1409	93.9
56000	58000	23	1.5	1432	95.5
58000	60000	22	1.5	1454	96.9
60000	62000	18	1.2	1472	98.1
62000	64000	15	1.0	1487	99.1
64000	66000	8	0.5	1495	99.7
66000	68000	5	0.3	1500	100.0
TOTAL		1500	100.0		

STATISTIX FOR WINDOWS **November**
 FREQUENCY DISTRIBUTION OF SUMFLOW

LOW	HIGH	FREQ	PERCENT	CUMULATIVE	
				FREQ	PERCENT
4000	6000	2	0.1	2	0.1
6000	8000	6	0.4	8	0.5
8000	10000	21	1.4	29	1.9
10000	12000	51	3.4	80	5.3
12000	14000	98	6.5	178	11.9
14000	16000	100	6.7	278	18.5
16000	18000	116	7.7	394	26.3
18000	20000	171	11.4	565	37.7
20000	22000	187	12.5	752	50.1
22000	24000	155	10.3	907	60.5
24000	26000	166	11.1	1073	71.5
26000	28000	128	8.5	1201	80.1
28000	30000	90	6.0	1291	86.1
30000	32000	70	4.7	1361	90.7
32000	34000	54	3.6	1415	94.3
34000	36000	38	2.5	1453	96.9
36000	38000	37	2.5	1490	99.3
38000	40000	9	0.6	1499	99.9
40000	42000	1	0.1	1500	100.0
TOTAL		1500	100.0		

STATISTIX FOR WINDOWS **August**
 FREQUENCY DISTRIBUTION OF SUMFLOW

LOW	HIGH	FREQ	PERCENT	CUMULATIVE	
				FREQ	PERCENT
10000	12000	3	0.2	3	0.2
12000	14000	3	0.2	6	0.4
14000	16000	1	0.1	7	0.5
16000	18000	1	0.1	8	0.5
18000	20000	5	0.3	13	0.8
20000	22000	3	0.2	16	1.0
22000	24000	16	1.0	32	2.1
24000	26000	24	1.5	56	3.6
26000	28000	35	2.3	91	5.9
28000	30000	82	5.3	173	11.2
30000	32000	88	5.7	261	16.8
32000	34000	113	7.3	374	24.1
34000	36000	78	5.0	452	29.2
36000	38000	90	5.8	542	35.0
38000	40000	92	5.9	634	40.9
40000	42000	112	7.2	746	48.1
42000	44000	177	11.4	923	59.5
44000	46000	145	9.4	1068	68.9
46000	48000	122	7.9	1190	76.8
48000	50000	105	6.8	1295	83.5
50000	52000	69	4.5	1364	88.0
52000	54000	49	3.2	1413	91.2
54000	56000	34	2.2	1447	93.4
56000	58000	38	2.5	1485	95.8
58000	60000	23	1.5	1508	97.3
60000	62000	34	2.2	1542	99.5
62000	64000	6	0.4	1548	99.9
64000	66000	2	0.1	1550	100.0
TOTAL		1550	100.0		

STATISTIX FOR WINDOWS **October**
 FREQUENCY DISTRIBUTION OF SUMFLOW

LOW	HIGH	FREQ	PERCENT	CUMULATIVE	
				FREQ	PERCENT
6000	8000	1	0.1	1	0.1
8000	10000	2	0.1	3	0.2
10000	12000	1	0.1	4	0.3
12000	14000	1	0.1	5	0.3
14000	16000	1	0.1	6	0.4
16000	18000	5	0.3	11	0.7
18000	20000	13	0.8	24	1.5
20000	22000	50	3.2	74	4.8
22000	24000	31	2.0	105	6.8
24000	26000	79	5.1	184	11.9
26000	28000	97	6.3	281	18.1
28000	30000	107	6.9	388	25.0
30000	32000	147	9.5	535	34.5
32000	34000	209	13.5	744	48.0
34000	36000	167	10.8	911	58.8
36000	38000	140	9.0	1051	67.8
38000	40000	127	8.2	1178	76.0
40000	42000	144	9.3	1322	85.3
42000	44000	99	6.4	1421	91.7
44000	46000	60	3.9	1481	95.5
46000	48000	34	2.2	1515	97.7
48000	50000	13	0.8	1528	98.6
50000	52000	8	0.5	1536	99.1
52000	54000	8	0.5	1544	99.6
54000	56000	6	0.4	1550	100.0
TOTAL		1550	100.0		

STATISTIX FOR WINDOWS **December**
 FREQUENCY DISTRIBUTION OF SUMFLOW

LOW	HIGH	FREQ	PERCENT	CUMULATIVE	
				FREQ	PERCENT
6000	8000	41	2.6	41	2.6
8000	10000	84	5.4	125	8.1
10000	12000	218	14.1	343	22.1
12000	14000	382	24.6	725	46.8
14000	16000	299	19.3	1024	66.1
16000	18000	188	12.1	1212	78.2
18000	20000	118	7.6	1330	85.8
20000	22000	69	4.5	1399	90.3
22000	24000	47	3.0	1446	93.3
24000	26000	41	2.6	1487	95.9
26000	28000	37	2.4	1524	98.3
28000	30000	18	1.2	1542	99.5
30000	32000	8	0.5	1550	100.0
TOTAL		1550	100.0		